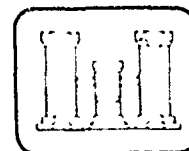


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## *Shallow snow model for predicting vehicle performance*

William L. Harrison

October 1981

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## PREFACE

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# LIST OF SYMBOLS

$A$	contact area ( $m^2$ )
$A_p$	peripheral area ( $m^2$ )
$B_1$	shear-stress/strain parameter ( $m^{-1}$ )
$b$	track or wheel width (m)
$C_c$	vehicle characteristic constant
$c$	internal shearing resistance (cohesion) (kPa)
$c_a$	interface shearing resistance (adhesion) (kPa)
$D$	drawbar pull
$d$	tire diameter (m)
$E$	compaction energy (J)
$F$	vertical force (N)
$f_i$	coefficient of friction
$g$	acceleration of gravity ( $m/s^2$ )
$H$	gross tractive effort (N)
$h$	snow depth or layer thickness (m)
$i$	slip ratio
$j$	slip distance (m)
$k$	contact length (m)
$l$	contact length (m)
$n_a$	number of axles on vehicle
$n_f$	maximum number of tires per axle
$p, p_z$	contact pressure (kPa)
$p_c$	carcass stiffness (kPa)
$p_i$	tire inflation pressure (kPa)

$p_z$	pressure at sinkage $z$ (kPa)
$R$	resistance to motion (N)
$R_c$	compaction resistance (N)
$R_t$	total resistance (N)
$r$	tire radius (m)
$V$	speed (m/s)
$V_s$	interface velocity (m/s)
$V_1$	initial volume ( $m^3$ )
$V_2$	final volume ( $m^3$ )
$W$	vehicle weight (kg)
$z_1, z_e, z$	sinkage (m)
$\alpha$	vehicle trim (degrees)
$\beta$	plastic kinematic viscosity ( $m^2/s$ )
$\gamma_1$	bulk density or specific weight ( $Mg/m^3$ )
$\gamma$	critical density ( $Mg/m^3$ )
$\gamma_i$	initial density ( $Mg/m^3$ )
$\gamma_0$	compacted density ( $Mg/m^3$ )
$\delta$	tire deflection (m)
$\theta$	slope or grade (degrees)
$\mu$	$\tan \phi$
$\rho$	angle of approach (degrees)
$\sigma$	normal stress (kPa)
$\sigma_t$	tensile strength of ice (kPa)
$\tau$	shearing strength (kPa)
$\tau_\phi$	dynamic shearing resistance (kPa)
$\phi$	angle of internal shearing resistance (degrees)
$\omega$	work of compaction per unit volume ( $J/m^3$ )
$\Phi$	angle of interface shearing resistance (degrees)

# SHALLOW SNOW MODEL FOR PREDICTING VEHICLE PERFORMANCE

William L. Harrison

## INTRODUCTION

The state-of-the-art of predicting vehicle performance in snow is still at the pioneering level. Although work has been done towards this end for at least 25 years, there has yet to appear in the literature a prediction model based on the treatment of snow as a unique material (Yong and Harrison 1978). It is anticipated that this situation will change in the next few years and attention will be given to the development of a proper snow failure model.

The purpose of this report is to review all past efforts that have been devoted to the prediction of vehicle performance in shallow snow layers and establish the current state-of-the-art. Within this context, a performance model will be recommended for use as a "best estimate" of the traction and resistance developed by wheeled and track-laying vehicles in shallow snow.

In this report, consideration will be given to the modification of traction and resistance forces by vehicle morphology and various types of traction aids. The snow cover considered will range in strength properties from those encountered in dry snow to those of slush layers, and the snow will be presumed to be deposited on a firm base. An assessment will be made of the accuracy of current methods and a model will be based on available test data from past exercises.

## HISTORICAL REVIEW

The literature relating specifically to vehicle mobility through shallow snow (ground support) is

not extensive. The problem has, however, been sparingly addressed in the last 25 years relative to vehicle performance predictions. It is generally agreed upon in the literature that the definition of shallow snow depends on the vehicle of interest. The snow-pack is said to be shallow when the disturbed volume (pressure bulb) beneath a traction element is in contact with the ground.

Within this context of vehicle performance in shallow snow, several different layer conditions must be considered. The first condition, which represents the idealized representation of shallow snow, is that of a homogeneous layer of settled snow covering a nondeforming subsurface. The second condition is that of a packed snow layer over a nondeforming subsurface.

Other surface conditions such as glare ice, slush, snow-ice, and thawing soil are of interest in this study but will be addressed later in this report.

Nuttall (1957) described a possible approach to predicting the resistance force acting on vehicles traveling in shallow snow layers. It will become apparent that many subsequent efforts bear some degree of similarity to this proposition. Nuttall's basic premise was based on pressure-density data which in general appearance is as shown in Figure 1.

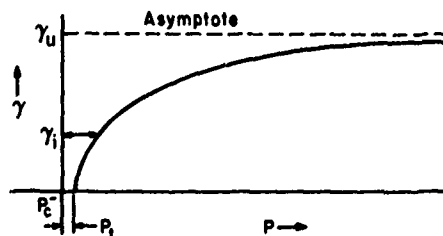


Figure 1. Pressure-density curves.

The relationships developed by Nuttall were an application of the Bernstein (1913) and Bekker (1952) equations for the work of compaction:

$$R = \int_0^z p \, dz \quad (1)$$

Nuttall expresses  $p$  in terms of density and obtains

$$\frac{R}{F} = \frac{h}{\ell} \frac{p_z}{p} \left(1 - \frac{\gamma_i}{\gamma_0}\right) \quad (2)$$

where  $R$  = snow resistance to motion (N)

$F$  = vertical force (N)

$R/F$  = dimensional quantity of resistance (N)  
per applied load (N)

$h$  = snow depth (m)

$\ell$  = contact length (m)

$p_z$  = pressure at sinkage  $z$  (kPa)

$p$  = average ground pressure (kPa)

$\gamma_i$  = initial snow density  $\text{g/cm}^3$

$\gamma_c$  = final snow density  $\text{g/cm}^3$

If eq 2 were expressed in terms of resistance per unit change in volume and density in terms of sinkage, it would take the following form:

$$R = bp_z z \quad (3)$$

(since  $p_z = F/b\ell$  and  $z = h[1 - (\gamma/\gamma_0)]$ ) where  $b$  is track or tire width and  $h$  snow depth. Equation 3 bears a close resemblance to the form postulated by Harrison (1975) which is discussed later.

Nuttall and McGowan (1961) collaborated on work using scale models of vehicles in snow to predict the performance of full-sized vehicles. The basic relationship developed and used to express performance was  $D/W$  vs the load numeric  $(W/c d^2)$ , where  $D$  is drawbar pull,  $d$  tire diameter,  $W$  vehicle weight, and  $c$  the structural cohesion. This relationship comes from the dimensional equations of the dependent variables of sinkage  $z$ , vehicle trim  $a$ , and drawbar pull  $D$ .

A significant feature of the work by Nuttall and McGowan (1961) is that, when establishing dimensional equations, much thought was given to the factors relating to the vehicle, to the snow, and to the system as a whole. The following listing from Nuttall and McGowan (1961) is considered worthwhile repeating as most of these factors must be considered in this study. Some items have been updated or modified by more recent research efforts.

#### Factors relating to the vehicle

1. Geometry or/and configuration, i.e. length, wheel base, dimensional aspects of traction elements, and number of propulsion units (articulated configuration, etc.).

2. Weight.

#### Factors relating to the operation of the vehicles

3. Speed, including slip.

4.  $j$  = slip distance of a tractive element relative to the undisturbed material during one cycle in contact with the material.

5.  $V$  = a characteristic speed related to the vehicle, such as peripheral wheel speed relative to the vehicle. At a given moment, all other velocities and components in the system may be expressed in terms of ratios to this speed. One ratio of particular importance in dynamic systems is the slip ratio  $i$ :

$$i = V_s / V \quad (4)$$

where  $V_s$  is the slip speed or average speed of the wheel contact area relative to the undisturbed soil or snow. It is to be noted that, while the slip ratio is usually conceived in terms of speed, it also has a simple geometric interpretation:

$$i = j/\ell \quad (5)$$

where  $j$  is defined in factor 4 and  $\ell$  is the distance from front to rear of the running gear contact area along the same path over which  $j$  is measured (usually parallel to the average surface of the material). In terms of speeds, the slip ratio implies dynamic snow reactions arising from inertial and/or viscous effects. In terms of distances, the ratio implies essentially static material reactions, determined primarily by displacement but little influenced by time-rate of displacement. (While both effects may be implied, dimensional analysis will not tell you which.)

#### Factors relating to the snowpack

6.  $h$  = total depth of material of interest. Boundaries of significant



layers, and hence thicknesses as well, may be specified by measurements from the surface.

7.  $c$  = before-collapse, or *structural cohesion*, of the material at depth  $h$ . This will, in general, vary at a given spot and time from layer to layer, and hence with depth.

8.  $\tau_\phi$  = the full, unit after-collapse, or *dynamic shearing resistance*, of the material originally at depth  $h$ . For any depth, it may be approximated by Coulomb's assumption:

$$\tau_\phi = c + \sigma \tan \phi \quad (6)$$

where  $\sigma$  is the unit normal loading on the shear plane. On this basis,  $\tau_\phi$  may be replaced by  $c$  and  $\tan \phi$ , where these are, respectively, the effective cohesion and the tangent of the effective angle of internal friction of the collapsed material originally at depth  $h$ .

9.  $\gamma$  = the before-collapse bulk specific weight of the material at depth  $h$ .

10.  $\beta$  = kinematic viscosity at depth  $h$ .

11.  $\tan \theta$  = slope of the surface of the material.

12.  $f_i$  = coefficient of friction of soils or snows at depth  $h$  in the material.

13.  $B$  = stress-strain parameter characterizing dynamic shearing resistance in the layer. Dynamic shearing resistance develops only after some consolidation and reorientation of the grains of the collapsed material. The typical relationship in weak, loose materials between unit shearing resistance and shear travel may be approximated by

$$\tau = \tau_\phi (1 - e^{-Bj}) \quad (7)$$

Because  $Bj$  must be dimensionless, the dimension of the parameter  $B$  is  $\ell^{-1}$ .

The list might be further lengthened by the inclusion of soil or snow elastic properties, but the phenomena of interest regularly involve large, permanent deformations of the material, so that elastic forces may be neglected from the outset, at least until experimental evidence of their importance dictates otherwise.

*Factors relating to the system as a whole*

14.  $g$  = acceleration of gravity.

#### *Dependent variables*

15.  $z$  = sinkage of vehicle.

16.  $\alpha$  = trim of vehicle

17.  $D$  = drawbar pull, or measurable margin of tractive capacity over external motion resistance. Drawbar pull of a given machine in a given material is largely influenced by slip ratio  $i$  and/or grouser travel  $j$ . Either drawbar pull or slippage may be taken as the independent variable, and the remaining one as dependent. In testing, it is convenient to control slippage and to measure drawbar output. In practice, the drawbar load is fixed, at any given moment, by terrain and towed load, and slippage becomes the dependent variable.

Most of these factors will appear in the algorithm for computing gross tractive effort and snow resistance to motion.

Blackmon and Rula (1960) presented a compilation of tests conducted in Boulder and Camp Hale, Colorado; Fort Churchill and Kapuskasing, Canada; and Houghton, Michigan. Some of the tests can be considered as shallow snow tests. Figure 2 shows snow depth vs rut depth of two Army vehicles operating at Camp Hale, Colorado: a M8E2 cargo tractor and a M5A4 high-speed tractor.

Plots of the performance of the M8E2 and the M5A4 (ground pressure  $\approx 42.7$  and  $57.9$  kPa, respectively) indicate that the compacted snow density beneath the ruts remained the same regardless of the original snow depth; i.e. rut depth = snow depth  $\times (\gamma/\gamma')$ . Assuming that the virgin snow density  $\gamma$  was constant, the compacted snow density  $\gamma'$  would also have to be constant in order that the slope of the curves in Figure 2 remain constant. Figure 3 (again from Blackmon and Rula 1960) shows the relationship between maximum drawbar-pull and snow depth for the M8E2 and the M5A4.

Figures 2 and 3 indirectly show the relationship between rut depth and resistance in shallow snow for the M8E2 and M5A4. Figure 4 (from Abele and Parrott 1968) gives a tractive coefficient for the same two vehicles of  $\tan \phi = 0.44$ .

The tests conducted at Houghton, Michigan, by Abele and Parrott (1968) did not include the same type of data as did tests at Camp Hale, and no assessment could be made of the two vehicles most likely to give shallow snow test results: namely the D4 and D7 tractors in approximately 46-cm snow depth. One very interesting result which comes from Table 24 of Abele and Parrott (1968) (a

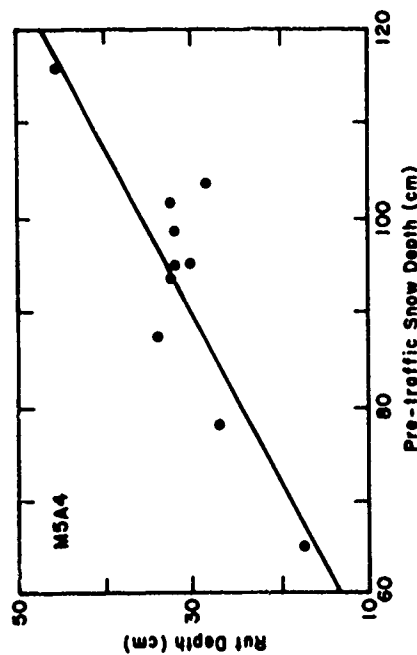
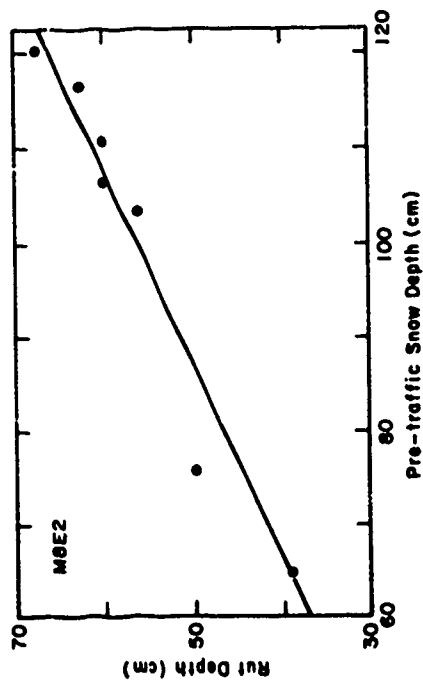


Figure 2. Self-propelled tests: snow depth vs rut depth, first test period, Camp Hale, Colorado (from Blackmon and Rula 1960).

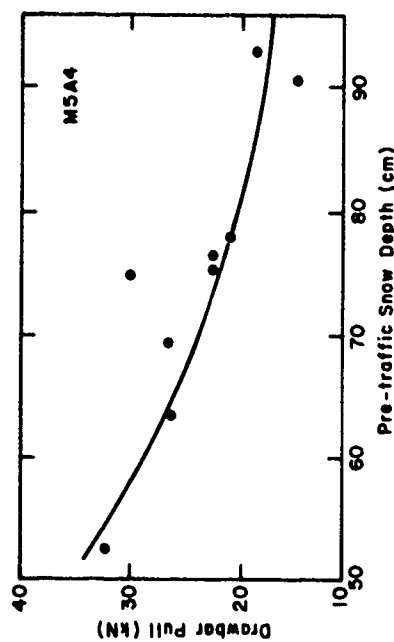
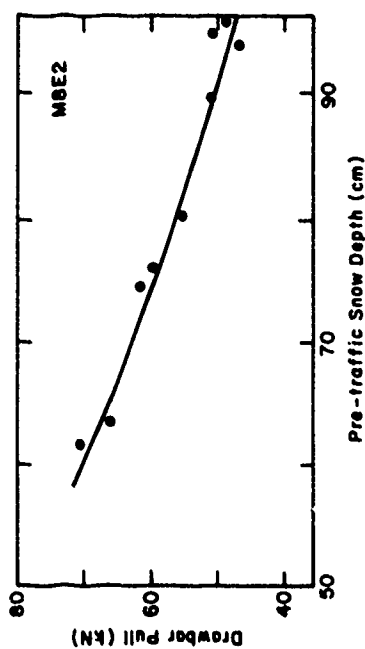


Figure 3. Snow depth vs maximum drawbar-pull: first test period, Camp Hale (from Blackmon and Rula 1960)

Table 1. Shallow snow data—snow characteristics.

Location	$T^*$ (°C)	$\gamma$ (g/cm <sup>3</sup> )	$h$ (cm)	$z$ (cm)	$d^\dagger$ (cm)	$\omega$ (joules/cm <sup>3</sup> )	$c_u$ (kPa)	$\Phi$ (degrees)	$\zeta$ (kPa)	$\phi$ (degrees)
Houghton	-7	0.15	14	10	20.3	0.30	0	18.8	1.5	18.2
30 Jan 75	-7	0.15	16	12	20.3	0.50	0	18.8	1.5	18.2
	-7	0.15	14	11	20.3	0.35	0	18.8	1.5	18.2
	-7	0.15	11	9	20.3	0.29	0	18.8	1.5	18.2
	-7	0.15	12	10	20.3	0.42	0	18.8	1.5	18.2
	-7	0.15	14	10	25.4	0.29	0	18.8	1.5	18.2
	-7	0.15	17	13	20.3	0.36	0	18.8	1.1	20.2
	-7	0.15	18	14	20.3	0.64	0	18.8	1.1	20.2
	-7	0.15	19	15	20.3	0.55	0	18.8	1.1	20.2
	-7	0.15	18	14	25.4	0.59	0.27	17.5	1.1	20.2
	-7	0.15	21	15	20.3	0.49	0.27	17.5	0.89	20.2
	-7	0.15	21	16	20.3	0.63	0.27	17.5	0.89	20.2
	-7	0.15	20	16	20.3	0.66	0.27	17.5	0.89	20.2
	-7	0.15	19	13	25.4	0.42	0	20.3	0.89	20.2
	-5	0.14	17	12	20.3	0.55	0	20.3	0.4	22.2
	-5	0.14	16	12	20.3	0.50	0	20.3	0.4	22.2
	-5	0.14	16	11	20.3	0.36	0	20.3	0.4	22.2
	-5	0.14	13	10	20.3	0.48	0	20.3	0.4	22.2
	-5	0.14	14	11	20.3	0.57	0	20.3	0.4	22.2
	-5	0.14	16	12	20.3	0.58	0	20.3	0.4	22.2
	-5	0.14	14	10	25.4	0.31	0	20.3	0.45	22.2
19 Feb 75	-12	0.26	11	7	20.3	0.60	0	22.8	0.13	25.9
	-12	0.26	11	8	20.3	0.91	0	22.8	0.13	25.9
	-12	0.26	11	8	20.3	0.60	0	22.8	0.13	25.9
	-12	0.26	10	7	20.3	0.70	0	22.8	0.13	25.9
	-12	0.26	11	6	25.4	0.40	0	22.8	0.13	25.9
	-12	0.26	12	6	25.4	0.60	0	22.8	0.13	25.9
	-4.5	0.23	14	9	20.3	0.28	0.34	25.2	0.69	24.7
	-4.5	0.23	11	9	20.3	0.58	0.34	25.2	0.69	24.7
	-4.5	0.23	12	5	20.3	0.38	0.34	25.2	0.69	24.7
	-4.5	0.23	9	6	25.4	0.26	0.34	25.2	0.69	24.7
3 Mar 75	-3	0.15	9	7	20.3	0.17	0.27	22.8		
	-3	0.15	10	7	20.3	0.28	0.27	22.8		
	-3	0.15	12	8	20.3	0.27	0.27	22.8		
	-3	0.15	7	6	25.4	0.05	0.27	19.4		
	-3		7	6	20.3	0.14	0.27	19.4		
	-3		8	6	20.3	0.21	0.27	19.4		
	-3	0.30	12	7	20.3	0.38	0	19.8		
	-3	0.30	12	9	20.3	0.50	0	19.8		
	-3	0.30	13	0	20.3	0.32	0	19.8		
	-3	0.30	16	10	25.4	0.35	0	19.8		
4 Feb 76			11	8	15.2	1.27	1.0	21		
			10	7	15.2	1.01	1.0	20		
			10	8	15.2	1.42	1.0	18		
			8	7	15.2	1.34	1.0	19		
5 Feb 76			9	7.7	15.2	1.61	1.0	21		
			11	9.3	15.2	1.24	1.4	21		
18 Feb 76			15	10	15.2	0.69	1.2	33		
			14	11	15.2	0.92	1.2	33		
			17	12	15.2	0.80	1.2	33		
			18	12	15.2	0.75	1.2	33		
19 Feb 76			16	13	15.2	1.33	0.68	24.7		
			16	13	15.2	1.32	0.68	24.7		
			15	12	15.2	1.39	0.68	24.7		

\* Snow temperature.

† Plate diameter.

Table 1 (cont'd). Shallow snow data—snow characteristics.

Location	T (°C)	$\gamma$ (g/cm <sup>3</sup> )	h (cm)	z (cm)	d (cm)	$\omega$ (joules/cm <sup>3</sup> )	ca (kPa)	$\phi$ (degrees)	c (kPa)	$\phi$ (degrees)
<i>Silver Creek Bridge*</i>	- 5	0.1	29.2	17	5.0	14.4†			0.44	20.8
30 Jan 73	- 5	0.1	29.2	15	7.6	9.00			0.44	20.8
	- 5	0.1	29.2	13	10.2	5.03			0.44	20.8
	- 5	0.1	29.2	9	12.7	2.80			0.44	20.8
	- 5	0.1	29.2	16	7.6	9.10			0.02	31.2
	- 5	0.1	29.2	15	10.2	5.65			0.02	31.2
	- 5	0.1	29.2	12	12.7	3.4			0.02	31.2
	- 5	0.12	25.4	15	7.6	10.33			0	27
	- 5	0.12	25.4	11	10.2	4.59			0	27
	- 5	0.12	25.4	9	12.7	2.30			0	27
	- 6	0.12	25.4	19	5.0	16.23			0.15	28.1
	- 6	0.1	25.4	15	7.6	8.39			0.15	28.1
	- 6	0.1	25.4	11	10.2	3.72			0.15	28.1
	- 6	0.1	25.4	11	12.7	3.68			0.15	28.1
	- 4	0.16	23.5	22	5.0	16.35			0.11	26.6
	- 4	0.16	23.5	20	7.6	7.23			0.11	26.6
	- 4	0.16	23.5	21	10.2	5.45			0.11	26.6
	- 4	0.16	23.5	22	12.7	4.03			0.11	26.6
	- 6	0.15	25.4	22	5.0	13.17			0	31.2
	- 6	0.15	25.4	23	7.6	7.33			0	31.2
	- 6	0.15	25.4	23	10.2	4.98			0	31.2
	- 6	0.15	23	20	12.7	4.17			0	31.2
	- 6	0.15	23	21	12.7	3.38			0	31.2
	- 8	0.14	40	29	7.6	12.18			0.27	23.5
<i>Ft. Greely</i>										
2 Feb 72	- 7	0.2	43	34	11	1.13			2.4	32
3 Feb 72	-12	0.2	51	48	11	1.08			1.6	34
14 Feb 72	-12	0.22	58	48	11	1.05			2.3	36
15 Feb 72	-10	0.22	51	39	11	0.92			1.6	32
<i>Ft. Wainwright</i>										
23 Feb 72	-10	0.2	61	58	11	1.57			2.3	34
24 Feb 72	- 8	0.2	59	56	11	1.31			0.75	38
28 Feb 72	-10	0.21	74	63	11	1.30			2.3	36
1 Mar 72	-10	0.21	56	51	11	1.62			1.3	34
<i>Camp Hale, Colorado</i>										
24 Feb 58 to	- 8	0.25	76	35		0.92				
7 Mar 58	- 7	0.27	77	58		2.26				
	- 6	0.24	74	49		1.99				
	-11	0.27	71	52		2.20				
<i>Wolf Creek Rd. California</i>										
21 March 80	- 1	0.36	14	-	24	2.11				
	- 1	0.36	14	-	24	1.85				
	- 1	0.36	25.4	12.9	24	1.86				
	- 1	0.38	22	9.3	24	2.28				
<i>Alta, Utah</i>										
26 March 80	0	0.13	6.3	-	24	2.35				
27 March 80	- 3	0.13	24.4	-	24	0.67				
	- 3	0.13	25.9	21	24	1.08				
30 March 80	- 7	0.095	26.7	19.5	24	0.972				
1 April 80	- 1	0.18	30.3	25.5	24	1.42				
**	- 1	0.18	22.9	13.7	20.32	0.720				
**	- 1	0.18	22.9	13.7	20.32	0.682				

\* Bennett (1974)

† Large values due to a compacted layer of snow 6 to 10 cm under new snow.

\*\* Plate test

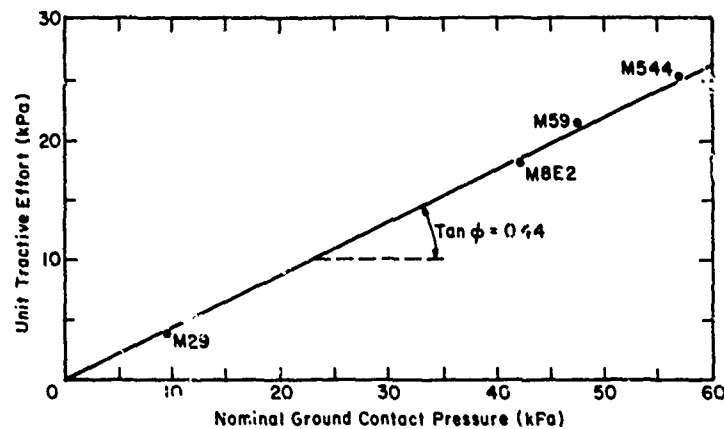


Figure 4. Comparison of nominal ground contact pressure and unit tractive effort.

summary of the Houghton tests) is support of the critical density theory of Gerdel et al. (1954), Marshall and Finelli (1955), and Harrison (1975). The after traffic density measurement of the first pass gave a value of approximately 0.40 for all vehicles regardless of ground pressure. This was true with only two exceptions: one with greater values (the F7 tractor) and the one with lower values (the M29 weasel). Unfortunately, these tests were the only ones from Abele and Parrish (1968) that could be considered as shallow snow tests.

A summary of the results of these tests from the viewpoint of vehicle performance in shallow snow is presented by S.J. Knight as Appendix C in CRREL Technical Report 268 (Harrison 1975). The conclusions were that, while the prediction techniques and equations developed from these data were suitable for predicting performance in deep snow, they were not applicable to the shallow snow problem.

An expedient solution for wheeled vehicles traveling through shallow snow was presented by Harrison (1973) for predicting motion resistance based on snow reaching a critical density of  $0.4 \text{ Mg m}^{-3}$ . Other pertinent assumptions were that the contact area for a wheeled vehicle would be the projected area under static load for a specific tire pressure. These data are usually available from the tire manufacturer.

The purpose of the approach presented in Harrison (1973) was to compute  $H_{\max}$  with no consideration given to slip or interface velocity.

To predict gross tractive effort  $H$  and motion resistance  $R$ , the following equations were suggested.

For gross traction:

$$H = W\mu + Ac' \quad (8)$$

where

$W$  = vehicle weight (or total load on powered wheels, kg)

$\mu = \tan \Phi$  where  $0 < \Phi < \phi$  (angle of interface friction)

$A$  = nominal contact area (static tire print for wheels,  $\text{m}^2$ )

$c' = c \frac{\tan \Phi}{\tan \phi}$  (adhesion)  $0 < c' < c$  ( $\text{kPa}^2$ )

For sinkage:

$$z = (1 - \frac{\gamma}{0.4}) h \quad (9)$$

where

$h$  = snow depth (m)

$\gamma$  = equivalent water content of snow (density in  $\text{Mg m}^{-3}$ ).

For compaction resistance:

$$R_c = \frac{bkz^{n+1}}{n+1} \quad (10)$$

$$R_c = 0.38 bp^{0.625} z^{1.975} \text{ at } n = 1.6$$

$b$  = track or tire width

$k = p/z^n$

$p$  = contact pressure

For bulldozing resistance:

$$R_b = (RN_w + \gamma z^2 N_{\gamma w} + cz N_{caw}) 2b \quad (11)$$

$$R = \gamma z^2 N_{\gamma} + cz N_{ca} \text{ (N/m)}$$

$\gamma$  = snow density ( $\text{Mg m}^{-3}$ )

$z$  = sinkage (m)

$c$  = snow cohesion (kPa)

$$\text{"N" factors} = f(\phi; \rho) \quad (12)$$

where  $\phi$  = snow internal friction angle

$\rho = (90^\circ + \text{approach angle of track})$  or  $[90^\circ + \arccos(z/\sqrt{z^2 + d^2}) \text{ for wheeled vehicles}]$

$d$  = tire diameter (m)

Equation 10 for compaction resistance is a modification of the deep snow tracked vehicle equation used by the Land Locomotion Laboratory (Harrison and Czako 1961). It was based on a selected value of  $n = 1.6$  which was representative of a number of experiments in deep snow.

Equation 11 for bulldozing resistance was adapted from the work of Reece (1965) and Hettiaratchi (1968). The curves for determining the "N" factors are contained in Hettiaratchi's work but are not considered relevant for inclusion or further discussion.

Liston (1974a, 1974b) presented an interesting approach that uses the sinkage of a plate which loads the snow cover to the same pressure as would a tire having a specified diameter, width and inflation pressure. An abridged version of his approach follows.

It is assumed that the maximum shear strength of snow can be described by the Mohr-Coulomb equation:

$$\tau = c + \rho \tan \phi \quad (13)$$

where  $\tau$  = shear stress (psi)

$\tau$  = shear stress (psi)

$c$  = apparent cohesion (psi)

$\rho$  = normal pressure (psi)

$\phi$  = angle of internal friction.

In determining the magnitude of  $c$  and  $\phi$ , a device having a rubber surface (rather than a roughened surface) is used to impose a failure surface within the snow mass. The more sophisticated equation of Hanamoto and Janosi (1961), which states that

$$\tau = (c + \rho \tan \phi) (1 - e^{-l/k}), \quad (14)$$

was not used because it was found that the magnitude of  $k$  was small, on the order of 0.02 m, so that maximum shear strength was developed along most of the tire/snow interface.

The contact area was assumed to consist of two sections: 1) an area  $A_p$  beginning at the uppermost contact point between the wheel and snow and ending at the bottom of the rut (see Figs. 5 and 6), and 2) an area  $A$  consisting of the flat contact patch at

the bottom of the tire.  $A_p$  is identified as the peripheral area and  $A$  as the contact area.

In order to compute the peripheral area  $A_p$ , it is noted that

$$dA_p = br d\theta \quad (15)$$

as shown in Figure 5, and

$$A_p = br \int_0^{\theta_0} d\theta = br \theta_0. \quad (16)$$

Also

$$\theta_0 = \cos^{-1} \left( \frac{r - z_0}{r} \right) \quad (17)$$

giving

$$A = br \cos^{-1} \left( \frac{r - z_0}{r} \right) \quad (18)$$

In computing the contact area ( $A$ ), it is assumed that

$$W = p_1 A \quad (19)$$

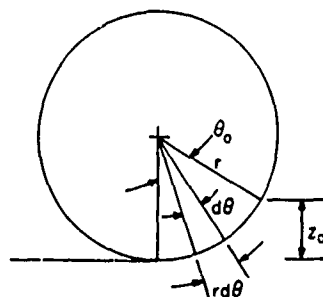


Figure 5. Determination of wheel/snow interface area (from Liston 1974a, 1974b).

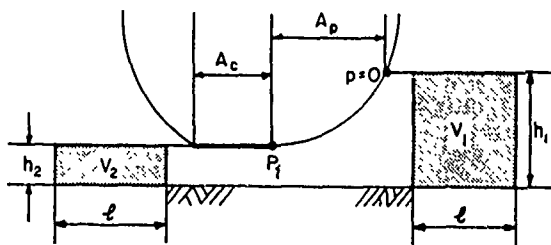


Figure 6. Volume change produced by passage of wheel and identification of contact areas.

where

$$\begin{aligned} p_i &= \text{inflation pressure (psi)} \\ A &= \text{contact area (in.}^2\text{)} \end{aligned}$$

Equation 19 is inaccurate as it does not account for the support provided by the tire carcass. The total contact area  $A_t$  per wheel is taken as

$$A_t = (A_p + A) \quad (20)$$

and gross traction is found from

$$H = (A_p + A) c + W \tan \phi. \quad (21)$$

In determining motion resistance, it was assumed that the work expended in compacting the snow was the single source of resistance offered by the snow. The following additional assumptions were made:

1. As shown in Figure 6, the only dimensional change that occurs when the snow is compacted from its original volume  $V_1$  to its final volume  $V_2$  is in the snow depth  $h$ .

2. The pressure along the interface of the wheel and snow is zero at the point of contact and is equal to the inflation pressure at the lowest point of contact.

3. The variation in pressure along the wheel/snow interface is hyperbolic in form. The pressure distribution is depicted in Figure 7.

The work expended in compacting the snow from  $V_1$  to  $V_2$  is

$$E = \int_{V_1}^{V_2} p \, dv. \quad (22)$$

The assumption that the pressure  $p$  varies hyperbolically leads to the following relationship:

$$p = p_i \frac{V_2}{V} \quad (23)$$

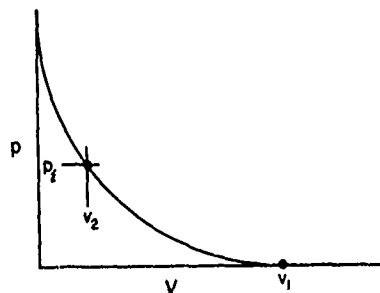


Figure 7. Assumed pressure distribution along wheel/snow interface.

in which

$$\begin{aligned} p_i &= \text{the inflation pressure} \\ V_1 &= bh_1\ell = \text{the initial volume of a snow mass} \\ V_2 &= bh_2\ell = \text{the final volume of the same snow mass} \end{aligned}$$

Introducing eq 24 into eq 23 and completing the integration gives the following expression for compaction energy  $E$ :

$$E = p_i V_2 \ln \frac{V_2}{V_1}.$$

If  $R_c$  is defined as the force resisting motion which is related to compaction, then work can be expressed by multiplying  $R_c$  by  $\ell$ . Also, the volume can be expressed as  $bh_1\ell$ . Substituting the above expressions for  $E$  and  $V$  gives the equation

$$R_c = p_i b h_2 \ln \frac{h_2}{h_1}. \quad (25)$$

The total motion resistance  $R_t$  is obtained by adding the compaction resistance to the hard surface rolling resistance  $R_H$ . In order to predict net tractive effort  $H_n$ , which is the parameter of interest, the motion resistance is subtracted from the gross traction:

$$H_n = H - R_H - 2(R_{c1} + R_{c2}) \quad (26)$$

in which  $R_{c1}$  and  $R_{c2}$  are the resistances of the front and rear wheels, respectively. Values of  $h_2$  are obtained from plate tests where the contact pressure of the plate and the tire are approximately equal.

Harrison (1975) presented a method for determining the motion resistance of vehicles in shallow snow based on the determination of a unit energy constant. The gross tractive effort is found by the use of a now-familiar expression:

$$H = A c_s + W \tan \phi \quad (27)$$

where  $A$  is the total contact area of the traction elements and  $W$  is the vehicle weight.

Motion resistance over shallow snow is defined as the work performed by the vehicle in compacting undisturbed snow when forming wheel ruts. Figure 8 shows the stress-deformation curve of a column of snow under simple compression.

The equation for work  $E$  is stated simply as

$$E = A \int_0^{z_c} \sigma \, dz \quad (28)$$

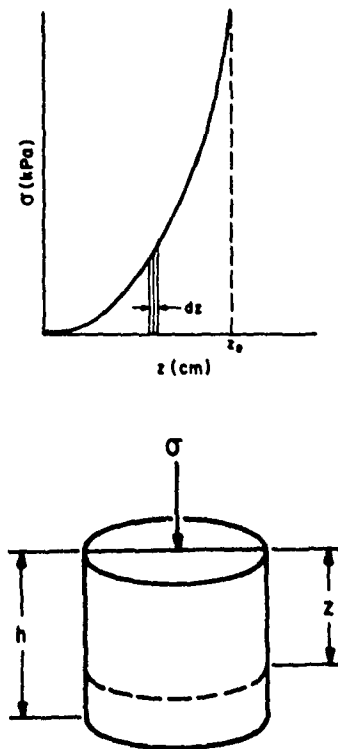


Figure 8. Column of snow under simple compression.

which is basically the same as those of Liston (1974), Bekker (1956), Nuttall (1957), etc. The characteristics of the curve differ from simple compression in a plastic medium in that there is compaction throughout deformation  $z$  rather than plastic flow as described by the typical soil model. In addition, the deformation  $z$  approaches a maximum value  $z_e$  as maximum compaction (critical density) is approached and the  $\sigma - z$  curve becomes asymptotic. Critical density is assumed to be approximately  $0.5 \text{ Mg m}^{-3}$ . This value of  $\gamma'$  is considered acceptable for two reasons: first, the additional deformation between  $\gamma = 0.5$  and  $\gamma = 1.0$  is minute and the increase in  $\sigma$  required is quite large, and secondly, in the unconfined state that exists, plastic flow will occur at this density rather than a decrease in volume (Kinoshita and Akitaya 1970).

The curves in Figure 9 illustrate the use of  $z_e$ . The effective sinkage ( $z_e$ ) is dependent on the depth at which  $\frac{1}{2} \gamma_{\text{max}}$  occurs and is easily computed from the following equation for predicting sinkage (Harrison 1973):

$$z_e = \left(1 - \frac{\gamma}{\gamma'}\right) h \quad (29)$$

where  $\gamma'$  is the selected value of post-compaction density. In practice, the measured curves are final-

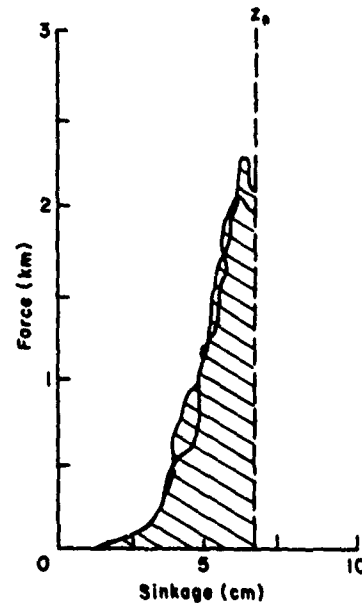


Figure 9. Plate sinkage tests (Harrison 1975).

ized (when necessary) by locating the  $z_e$  line and constructing the finished curve so that it becomes asymptotic with  $z_e$  (Fig. 9).

The area beneath the load-sinkage curve in Figure 9 represents the total work  $E$  expended in compacting the snow from  $z = 0$  to  $z = z_e$ . To obtain the work of compaction per unit volume  $\omega$  the total work  $E$  is divided by the snow depth  $h$  and the area of the circular plate  $A_p$  used to obtain the curves:

$$\omega = \frac{E}{h A_p} \quad (30)$$

To minimize the effect of size, a minimum plate diameter of 20 cm is used.

The resistance to motion from snow compaction is given by the equation

$$R_c = 2b\omega h \quad (31)$$

where  $b$  is the effective wheel width (or track width).

Bekker (1976) presented a method of determining rolling resistance based upon the shallow snow tests conducted by Harrison (1975). The method of solution combined the earlier efforts of Bekker and Semonin (1975) for determining the rolling resistance due to tire flexing and those of Bekker (1973) for determining the resistance to motion by vehicles traveling in a layered medium.

To determine the snow resistance to motion, Bekker utilizes a multi-layered approach based on the characteristics of plate sinkage curves in shallow



snow. As shown in Figure 10, the  $p - z$  curve is plotted in log-log form and the "zones" are selected dependent on obvious changes in slope " $n$ ." Each zone produces a set of  $k_c$ ,  $k_\phi$ , and  $n$  values by which the resistance for that zone can be calculated.

For "zone 1" where  $n$  is generally less than 3.0 or for any zone where  $n < 3.0$ , the resistance is determined by

$$R_c = \frac{b_0 k z_1^{(n+1)}}{n+1} \quad (32)$$

where  $z$  is determined from the expression:

$$z_1 = \left( \frac{3W_1}{bk\sqrt{D}(3-n)} \right)^{\frac{2}{2n+1}} \quad (33)$$

From Bekker (1976)

$$W_1 = 0.3 b_0 k (3-n) \sqrt{D} z_1^{\frac{(2n+1)}{2}} \quad (34)$$

Whenever there is a zone where  $n > 3.0$ , the "full solution of the integral  $W = b_0 \int_{z_0}^z p_x dx$  must be used" (Bekker 1976) so that

$$W_j = b_0 k_j \sqrt{D} \left[ 2_j \frac{2n_j + 1}{2} \left( a_j - b_j \frac{2z_j}{D} \right) - z_j - 1 \frac{2n_j + 1}{2} \left( a_j - b_j \frac{hz_j - 1}{D} \right) \right]$$

where

$$a_j = (1 - 0.509 n_j + 0.222 n_j^2 - 0.052 n_j^3 + 0.005 n_j^4) \quad (36)$$

and

$$b_j = (0.25 - 0.26 n_j + 0.137 n_j^2 - 0.028 n_j^3) \quad (37)$$

$R_c$  is then the sum of  $R_1, R_{11}, \dots, R_j$  and the total resistance for a wheeled vehicle is

$$R = 2(R_c + R_t) + n_w R_t \quad (38)$$

where  $n_w$  is the number of trailing wheels, i.e. 2 for a 4 x 4, 6 for an 8 x 8, etc.

Nuttall et al. (1975) developed an ad-hoc shallow snow model for wheels and tracks. The model is used as part of the Army Mobility Model to predict traction and resistance of a snow cover deposited on frozen soil.

The model computes maximum traction  $H$  and resistance  $R$ . Net traction is determined as the difference between  $H$  and  $R$ :

$$H = cA + W\mu \quad (39)$$

where

$c$  = interface shearing resistance (psi)  
 $A$  = contact area (in.<sup>2</sup>)  
 $W$  = vehicle weight (lb)  
 $\mu$  = tangent of the angle of interface shearing resistance.

For wheels the following expression is used:

$$\frac{R}{W} = \frac{10}{n_a} \frac{n_t b}{d} \frac{\gamma h}{\ell} \quad (40)$$

where

$\ell = 2 \sqrt{\delta d - \delta^2}$  (in.)  
 $d$  = tire diameter (undeflected) (in.)

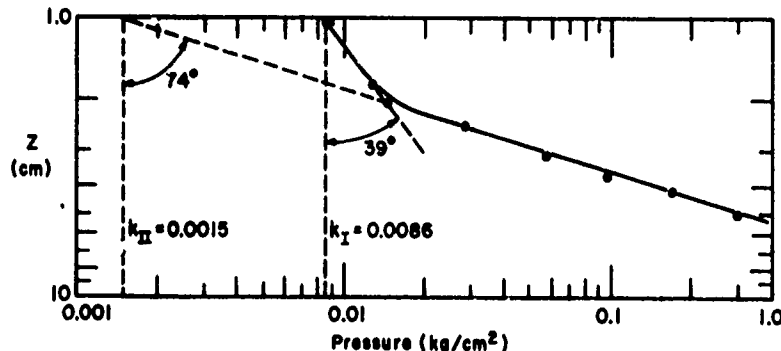


Figure 10. Pressure-sinkage graph of load-sinkage function in shallow snow. Snow depth is approximately 9 cm, temperature is  $-1^\circ\text{C}$ , and plate diameter about 20.3 cm (adapted from Harrison's 1975 data).

$\delta$  = tire deflection (in.)  
 $n_t$  = maximum number of tires per axle  
 $n_a$  = number of axles  
 $b$  = tire section width (in.)  
 $h$  = snow depth (in.)  
 $\gamma$  = snow specific weight

For tracks, Nuttall (1957) uses a simpler expression

$$\frac{R}{W} = \left( \frac{h}{L} - 0.15 \right) \quad (41)$$

where  $L$  = overall track length (in.)

The relationships were developed by Nuttall from a compilation of experimental test data available in the literature.

Table 2 gives a comparison of predictions by four models. These predictions were made from past data relative to vehicle tests in shallow snow as presented in Liston (1974a, 1974b), Harrison (1975) and Nuttall (1975). The prediction by the Bekker model was furnished the author by private communication with Dr. Bekker.

The high value produced by the Liston model for the Hanover tests results from the model's characteristic of inflation pressure dependency.

It can easily be shown that the Liston (1974), Bekker (1976), and Nuttall (1957) models (since all are compaction energy based) can be resolved into eq 31,  $R_c = 2b\omega h$ , which is the Harrison (1975) model with the parameter  $\omega$ , the work of compaction per unit volume, as the basic transfer mechanism.

## MODEL SELECTION

If the model to be proposed in this report is to be

selected on review of the past work, which must be considered as the state-of-the-art, then a number of guidelines or goals should be contemplated.

Before facing this selection process, let us review some common aspects of the past work. All methods discussed in the review proposed the use of some form of the Mohr-Coulomb relationship for predicting the tractive force developed by a vehicle. Further, most methods proposed that the energy dissipated in making the rut in snow be considered as the resistance to motion caused by compacting the snow. The most apparent weakness of all the methods is lack of validation by field tests.

The guidelines for model selection under the above-stated circumstances should therefore be based on the following:

1. The equations for traction and snow resistance to motion should clearly address the problem. These equations should be sufficiently fundamental that future modification is possible.
2. The parameters which reflect snow strength characteristics should facilitate prediction from basic snowpack properties and be obtainable by field measurement without great difficulty.
3. Because of the lack of a data bank on shallow snow-vehicle performance parameters, any proposed model must be considered provisional at best.

## Traction

The prediction of gross tractive effort is based on the Mohr-Coulomb expression:

$$\tau = \sigma \tan \phi + c$$

where  $\tau$  is shearing stress (kPa),  $\sigma$  is normal stress (kPa),

Table 2. Comparison of prediction models.

Vehicle	Location	Computed values of $R$ (N)*				Measured values (N)		
		Liston	Harrison	Nuttall	Bekker	$R$	$R_H$	$R_c$
			1975	1975				
M151A1	Silver Creek Bridge (30 Jan 73)	2594	2280	1584		2520†	373†	2146
Chevrolet	Hanover, N. H.	1628	636	609	540	1237†	657†	580
Carroll	(18 Feb 75)							
M151A1	Houghton, Mich.	898	884	1365		1490**	636**	853
	(18 Feb 76)							
M151A1	Houghton, Mich.	862	643	676		1365**	636**	729
	(30 Jan 75)							

\* Compaction resistance  $R_c = R - R_f$

† Measured by deceleration method

\*\* Measured by towing

and  $c$  and  $\phi$  are internal shearing resistance and the associated angle of shearing resistance, respectively. As used in most vehicle performance models, this equation is written in the form for predicting gross tractive force  $H$ :

$$H = Ac + W \tan \phi \quad (43)$$

for vehicles with chains or grousers. For vehicles with pneumatic tires or band tracks with no grousers,

$$H = A c_a + W \tan \phi \quad (44)$$

where  $A$  and  $W$  are the contact area and vehicle weight, and  $c_a$  and  $\theta$  are the interface shearing resistance and associated angle of interface shearing resistance, respectively. The prediction of gross traction  $H$  will be in terms of maximum sustained force rather than a traction-slip function.

To accommodate traction aids in the model, increased traction will be reflected by multiplying the results of a standard tire by coefficients obtained from experimental tests (Table 3) of Liston (1977).

The determination of total traction developed will be a summation of the traction developed by each traction element.

#### Resistance

The energetics relationship (Harrison 1975) which states

$$R_c = 2 b \omega h \quad (31)$$

is proposed as the resistance model. The relationship between  $\omega$ , snow temperature  $T$ , and snow density  $\gamma$  shown in Figure 11 requires further validating by a more complete data bank than shown in Tables 1 and 4, on which Figure 11 was based.

#### Slush and thawing soils

An ad-hoc model is suggested for slush and thawing soils until a data bank has been established. These two

materials, due to their characteristics of flowing rather than compacting when disturbed, will be treated as surcharge models having hydrostatic properties relative to the forces required to cause flow. It is assumed that the traction is obtained from the firm supporting layer. The base properties for slush will be packed snow at  $-1^\circ\text{C}$  (ice), and a layer having a high degree of unfrozen water for thawing soil. The resistance force is determined as follows:

$$R_c = \frac{\gamma h^2}{2} \quad (45)$$

where  $\gamma$  is the density of the layer and  $h$  is the layer thickness. Values of  $\gamma$  are as follows:

slush:  $\gamma = 0.75 \text{ g/cm}^3$  ( $750 \text{ kg/m}^3$ )

thawing soil layers:  $\gamma = 1760$  to  $2100 \text{ kg/m}^3$

with loams on the lower end and sands at the higher values. This relationship is loosely based on the unlikely premise that there will be no sinkage or break-through in the supporting layer.

#### Ice, hard-packed snow, packed snow (new)

These surface conditions are assumed to have negligible motion resistance. The emphasis in predicting performance is placed on the following equations:

$$H = W \tan \phi + Ac \quad (43)$$

or

$$H = W \tan \Phi + Ac_a \quad (44)$$

where  $\mu$  ( $\tan \phi$  or  $\tan \Phi$ ) in this context is determined from experimental data. Figures 12 and 13 show values of  $\mu$  tabulated from a large number of experiments, as functions of bearing capacity and snow surface temperature. Figure 14 gives values of  $\mu$  for ice relative to surface temperature. A summary of Figures 12, 13, and 14 is given in Table 5.

Table 3. Traction aid data

Tire	Pressure (kPa)	Drawbar pull (N)	Percent increase over NDCC standard	Traction aid coefficient
NDCC standard	55	2420	0	1
NDCC standard with chains	103	3372	60	1.60
NDCC radial	55	2535	25	1.25
Bias-ply snow	55	3056	35	1.35
Bias-ply snow with studs	55	3252	45	1.45
Summer radial	55	3016	45	1.45
Sand tire	55	2304	25	1.25

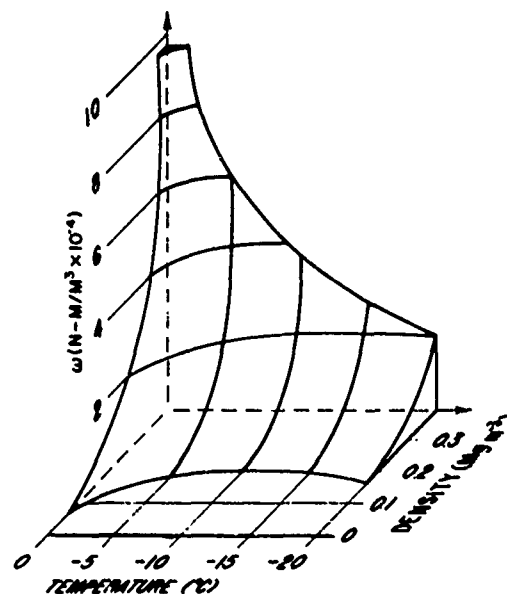


Figure 11.  $\omega$  versus temperature and density.

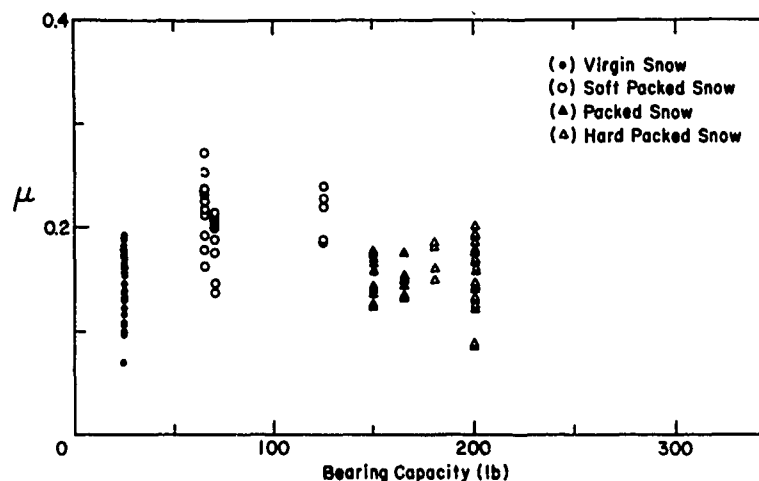


Figure 12.  $\mu$  versus bearing capacity—disturbed snow.

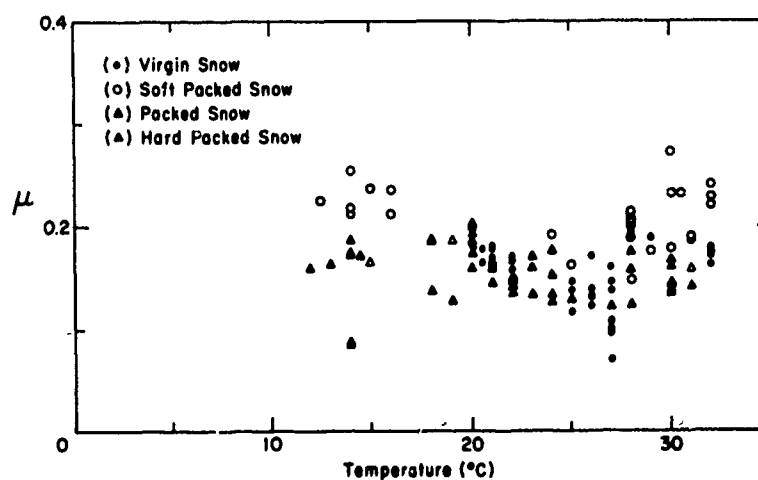


Figure 13.  $\mu$  versus temperature—disturbed snow.

Table 4. Shallow snow data - vehicle characteristics.

Location	Vehicle type	h (cm)	d (cm)	b (cm)	W (Kg)	P (kPa (psi))	D (N)	R (N)
<i>Houghton</i>								
30 Jan 75	M151	15	75	20	1,410	137 (20)	1,560	1,312
		13	75	21	1,410	103 (15)	1,640	1,267
		15	75	20	1,410	103 (15)	1,470	1,036
		13	75	22	1,410	55 ( 8)	2,090	1,365
		14	75	21	1,410	55 ( 8)	1,870	1,352
19 Feb 75	M151	8	75	22	1,410	103 (15)	1,380	720
		16	75		1,410	103 (15)		1,468
		8	75	23	1,410	55 ( 8)	1,650	854
3 Mar 75	M151	12	75	21	1,410	103 (15)	2,490	1,316
		12.7	75	22	1,410	55 ( 8)	2,220	1,556
12 Feb 75	M151	14	75	22	1,410	103 (15)	2,000	1,321
		15	75	24	1,410	55 ( 8)	2,220	1,374
4 Feb 76	ENGESA	10	775 <sup>1</sup>			172 (25)		3,430
5 Feb 76	2½-ton	14	775 <sup>1</sup>			172 (25)		4,400
18 Feb 76	M151A1	18	353 <sup>1</sup>		1,560	55 ( 8)		1,490
		14	353 <sup>1</sup>		1,560	55 ( 8)		1,490
19 Feb 76	ENGESA 2½-ton	16	775 <sup>1</sup>			172 (25)		3,700
<i>Ft. Greely</i>								
2 Feb 72	M113A1	43	280 <sup>2</sup>	38	9,500	43.8 (6.3)	34,000	
3 Feb 72		51	280 <sup>2</sup>	38	9,500	43.8 (6.3)	39,000	
14 Feb 72		58	280 <sup>2</sup>	38	9,500	43.8 (6.3)	38,000	
15 Feb 72		51	280 <sup>2</sup>	38	9,500	43.8 (6.3)	40,000	
23 Feb 72		61	280 <sup>2</sup>	38	9,500	43.8 (6.3)	32,000	
24 Feb 72		58	280 <sup>2</sup>	38	9,500	43.8 (6.3)	34,000	
28 Feb 72		74	280 <sup>2</sup>	38	9,500	43.8 (6.3)	29,000	
1 Mar 72		56	280 <sup>2</sup>	38	9,500	43.8 (6.3)	31,000	
<i>Silver Creek Bridge</i>								
30 Jan 73	M151A1	24	75	19	1,452	55 ( 8)	2,320	1,214
		24	75	19	1,452	103 (20)	1,630	2,520
27 Jan 73	M151A1	24	75	19	1,452	103 (20)	2,380	1,970
31 Jan 73	M34		107	29	7,720	241 (35)	3,400	790
<i>Camp Hale, Colorado</i>								
24 Feb 58 to	M29	76	198 <sup>2</sup>	51	1,950	9.6 (1.4)	5,100	5,700 <sup>3</sup>
7 Mar 58	M5A4	77	300 <sup>2</sup>	31	9,350	58 (8.4)	22,000	31,808
	M59	74	235 <sup>2</sup>	53	17,600	48 (7.0)	56,000	29,100
	M8EZ	71	406 <sup>2</sup>	53	18,900	41 (6.0)	58,000	35,500

<sup>1</sup> Actual tire contact area (cm<sup>2</sup>)

<sup>2</sup> Track length (cm).

<sup>3</sup> Vehicle dragging.

Table 5. Mechanical properties for ice and packed snow.

Parameter	Ice	Packed snow
tan $\phi$	0.07	0.23
tan $\phi$	0.03	0.14
c (kPa)	0	0.3
c <sub>2</sub> (kPa)	0	0.6

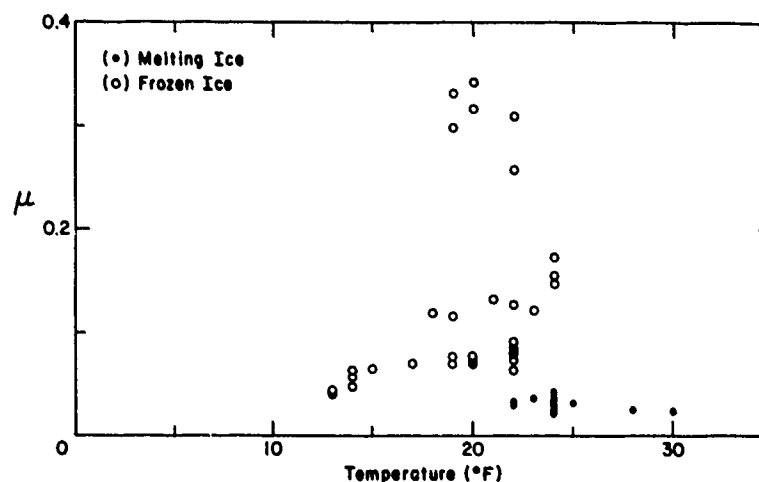


Figure 14.  $\mu$  versus temperature - ice.

Within the temperature ranges of the experiments, the data scatter was significant with no definite  $\mu$  vs temperature relationships apparent. If data were available for temperature ranges well below 0°F, perhaps some trends would have been evident.

The contact area  $A$  for pneumatic tires will be determined by the tire inflation pressure as follows:

$$A = \frac{W}{P_g} \quad (45)$$

where  $W$  is the wheel load and  $P_g$  is the tire inflation pressure, and by the width-length product for tracked vehicles:

$$A = 2b\ell \quad (46)$$

where  $b$  and  $\ell$  are the nominal width and length measurements.

Recommendation for use of eq 45 is based on Table 6 which shows values of measured tire contact areas divided by wheel load vs predictions using this equation. The average error is 10% for eq 45 over a number of inflation pressures and tire structural characteristics. A 10% average error is quite acceptable at this time.

#### River and lake ice

The safe crossing thickness of river or lake ice, i.e. freshwater ice, can be determined from the following equation (Johnson 1979):

$$h = C_c \sigma_t^{-0.6} \quad (47)$$

where  $h$  = ice thickness,  $C_c$  = vehicle characteristic constant and  $\sigma_t$  = tensile strength. A reasonable tensile strength for ice during winter conditions is 980 kPa (Nevel 1978), and in springtime this value deter-

iorates to 100 kPa.

Johnson (1979) gives the following values of  $C_c$  for the M151, M35A2, M113 and M60A1 Army vehicles. All values are for highway or combat loads: M151—8.1 kPa, M35A2—22.4 kPa, M113—23.0 kPa, M60A1—51.7 kPa. Thus, for example, during winter conditions, the safe thickness of ice for the vehicles as listed will be: M151—0.13 m, M35A2—0.36 m, M113—0.37m, M60A1—0.83 m.

#### Model use

The shallow snow model consists of the relationships given eq 42-47. Where possible, values of surface strength parameters have been, or will be, furnished from experimental tests. When this is not possible, the values will be predicted. Toward this end, a preliminary study was made (Berger in prep.). These studies will be later expanded in scope to include the forecasting of all parameters including packed snow, slush, thawing soils and ice.

Values of  $H$  and  $R$  are required by the Army Mobility Model (Nuttall et al. 1975). The decision-making process for obtaining these values by using eq 42-46 (and the associated tables) is in fact the shallow surface layer portion of the eventual "Cold Regions Mobility Model." This is intended for use as a submodel to the Army Mobility Model and associated models. The following flow diagrams and examples present an algorithm for the decision-making processes for determining  $H$  and  $R$ .

Factors to be considered in the algorithm are:

1. Number and arrangement of traction elements (i.e. driven, towed, duals).
2. Characteristics of traction elements: (i.e. smooth "rubber," grousered, inflation pressure, chains, studs, ply design) (see Table 3).
3. Surface material: (see Tables 5 and 7).

Table 6. Tire contact area (in.<sup>2</sup>).

Measured	Theoretical	$\Delta$	% Error
52.6	53.9	1.3	2.5
45.2	49.3	4.1	9.1
46.1	49.3	3.2	6.9
50.3	53.9	3.6	7.2
49.5	49.3	+0.2	0.4
52.6	53.9	1.3	2.5
45.0	49.3	4.3	9.6
49.5	53.9	4.4	8.9
44.0	49.3	5.3	12.0
48.5	53.9	5.4	11.1
50.3	53.9	3.6	7.2
50.9	53.9	3.0	5.9
54.2	53.9	+0.3	0.6
49.2	53.9	4.7	9.6
49.3	53.9	4.6	9.3
46.2	53.9	7.7	16.7
41.5	49.3	7.8	18.8
48.1	53.9	5.8	12.1
47.0	53.9	6.9	14.7
45.7	53.9	8.2	17.9
46.8	53.9	7.1	15.2
45.1	53.9	8.8	19.5
48.0	53.9	5.9	12.3
Avg.			10.0%

Actual programming of the model has not yet been undertaken. A description of the model can be divided into two portions, the input data structure and the various subprograms which calculate traction and motion resistance.

An explanation of the data structure involves a description of all the pertinent variables and how they are input into the model. The computer model breaks the vehicle down into a series of units. Each transverse set of axles or set of tracks is considered a unit. The variables pertaining to the geometry of wheeled and tracked vehicles are shown in Figures 15 and 16 respectively. Data for particular tire or track characteristics are indexed into a two dimensional array. Each of the vehicle units is represented by a row in the array and the columns indicate location on the unit.

For calculating traction, separate models have been developed for tracked and wheeled vehicles. The traction subprogram (Fig. 17 and 18) simply sums the traction values for each driven unit on the vehicle.

Resistance is calculated by the motion resistance subprogram (Fig. 19). It is based on eq 31 and uses  $\omega$  values.

No driver program has been assembled to coordinate the various subprograms. Basically, the driver program should merely establish a user interface for data input and output and call each of the subprograms to perform its function.

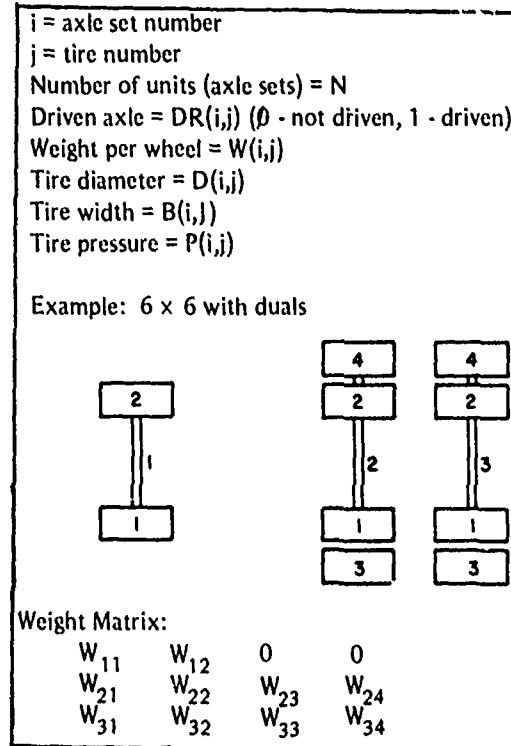


Figure 15. Inputs for wheeled vehicles.

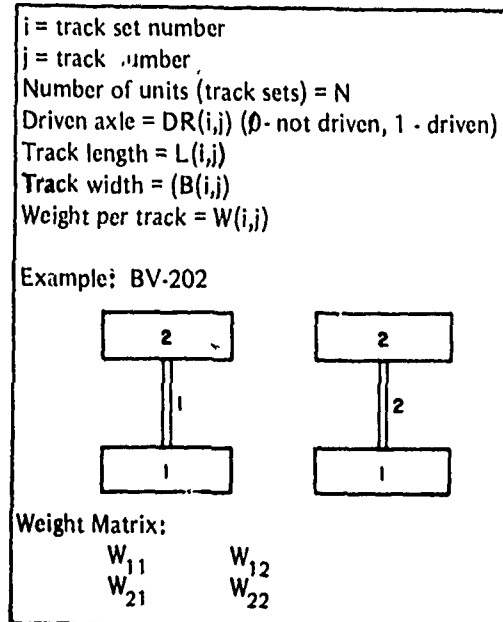


Figure 16. Inputs for tracked vehicles.

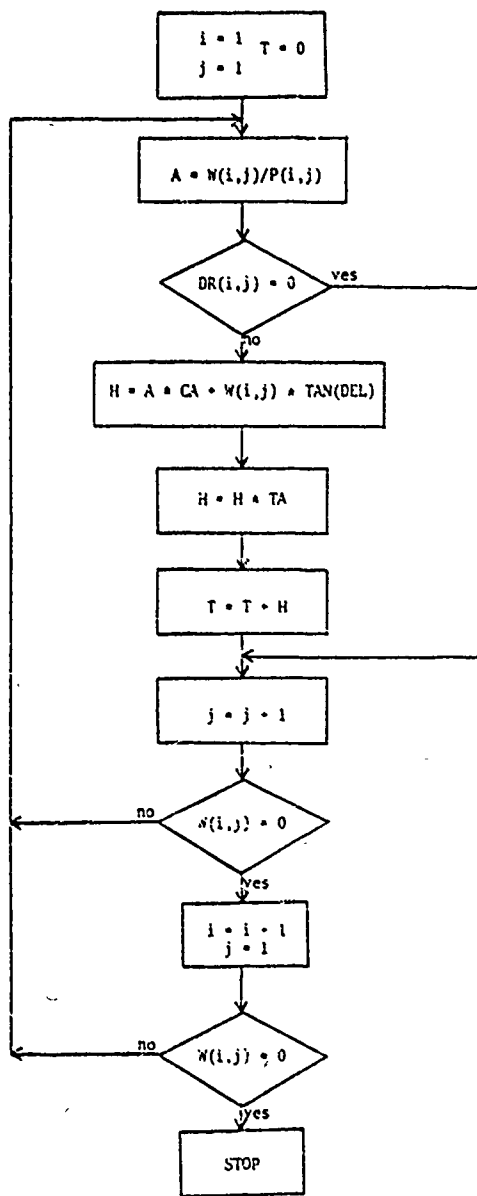


Figure 17. Traction subprogram for wheeled vehicles.

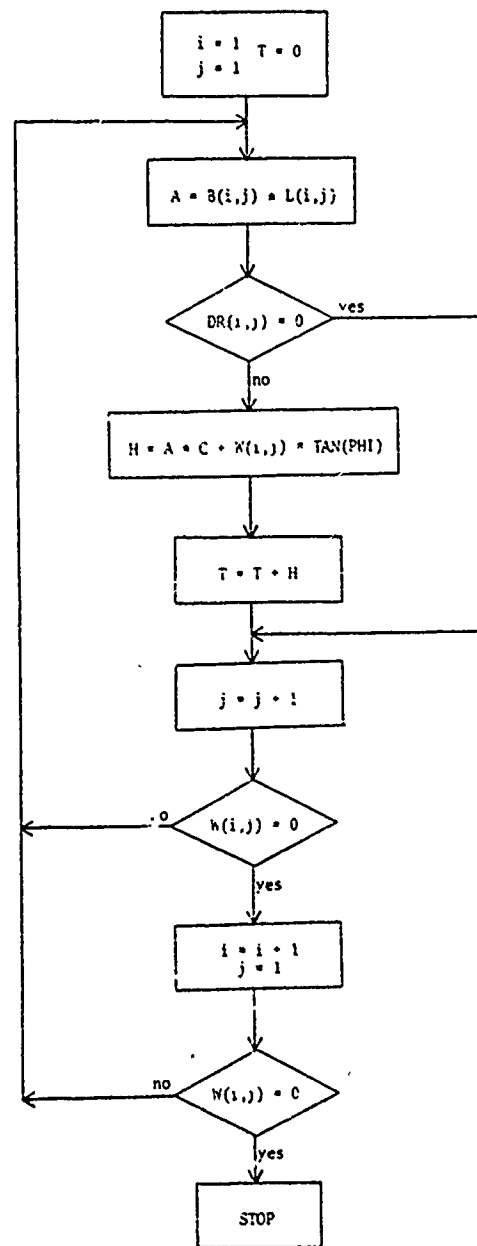


Figure 18. Traction subprogram for tracked vehicles.



Table 7. Mechanical properties of shallow snow.

Snow type	Snow temp ( $^{\circ}\text{C}$ )	Air temp ( $^{\circ}\text{C}$ )	$\rho$ ( $\text{g/cm}^3$ )	$c_a$ (kPa)	$c$ (kPa)	$\delta$ (degrees)	$\phi$ (degrees)	$\omega$ ( $\text{J/cm}^3$ )	$\rho$ (critical) ( $\text{g/cm}^3$ )
New Snow									
Dry <sup>1</sup>	-5 to -10	-9	0.15	0	1 to 1.5	18 to 20	18 to 20	0.4 to 0.6	
Dry <sup>4</sup>	-10 to -20	-15	0.2 to 0.26	0	0.5	22 to 24	26	0.7 to 0.8	
Wet <sup>5</sup>	-1 to -3	-1		1 to 1.2	—	25 to 30	—	1 to 1.2	
Dry <sup>a</sup>	-3	-6	0.13					0.67 to 1.08	
Dry <sup>b</sup>	-7	-10	0.095					0.972	
Settling snow									
Powder <sup>1</sup>	-3 to -5	-8	0.1 to 0.2	0	3.9	20	22	0.75	
Bonded <sup>2</sup>	-3 to -5	-7	0.3	0	—	20	—	0.6	
Bonded <sup>3</sup>	-3 to -5	-15	0.23		6.8	25	25	0.7	
Powder <sup>c</sup>	0	-2	0.13					2.35	
Powder <sup>d</sup>	-1	0	0.18					0.68 to 0.72	
Powder <sup>e</sup>	-1	0	0.18					1.4?	
Old snow									
Crusted <sup>6</sup>	-10	-13	0.21		0		20		
Crusted <sup>6</sup>	-5	-10	0.2		0.68		20		
Compacted <sup>6</sup>	-4 to -10	-1	0.4		3.0		20	~ 6	
Compacted <sup>6</sup>	-4 to -10	+6	0.4		0		30	~ 4	
Corn snow	0	>0	0.6		0				
Slush	0	>0	0.7		0				
Crusted <sup>f</sup>	-1	+3	0.36					1.85 to 2.11	
Crusted <sup>g</sup>	-1	+3	0.38					2.28	

<sup>1</sup>Houghton, 30 Jan 75.

<sup>2</sup>Houghton, 3 Mar 75.

<sup>3</sup>Alaska, Ft. Greely and Ft. Wainwright, Feb 72.

<sup>4</sup>Houghton, 4 Feb 76, 5 Feb 76.

<sup>5</sup>Houghton, 18 Feb 76.

<sup>6</sup>Silver Creek Bridge, Feb 73.

<sup>a</sup>Alta, Utah, 27 March 80 (vehicle test)

<sup>b</sup>Alta, 30 March 80 (vehicle test)

<sup>c</sup>Alta, 26 March 80 (vehicle test)

<sup>d</sup>Alta, 1 April 80

<sup>e</sup>Alta, 1 April 80 (vehicle test)

<sup>f</sup>Wolf Creek, California, 21 March 80 (vehicle test)

<sup>g</sup>Wolf Creek, 21 March 80 (vehicle test)

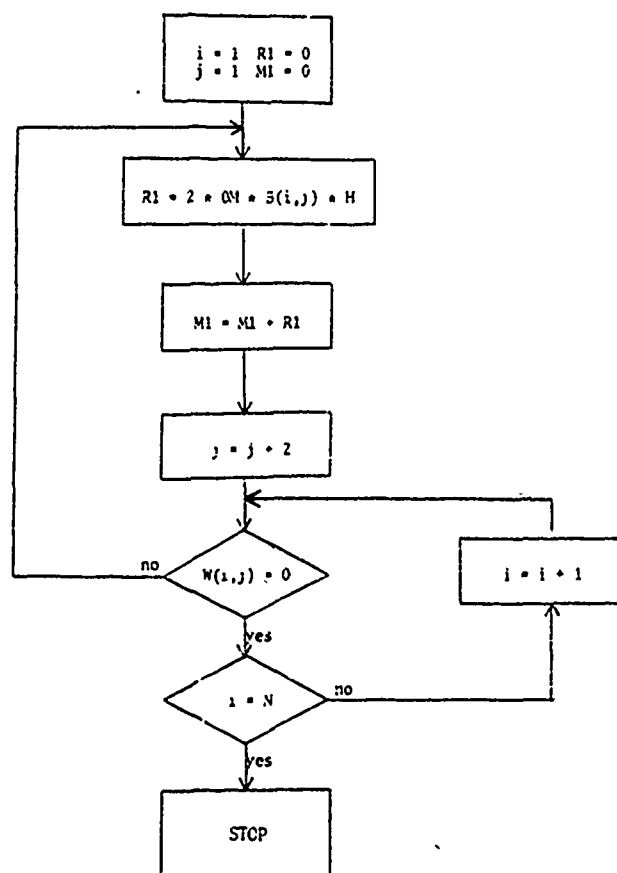


Figure 19. Motion resistance subprogram.

## CONCLUSIONS

Most tests and measurements reported were conducted under conditions as they occurred during a scheduled test period. This led in most cases to results that were nongeneric to the terminology used; i.e. shallow snow, packed snow, etc. In most cases, the shallow snow layers were complex in nature and varied considerably from site to site. The scatter in ice and packed snow data is also indicative of variations not accounted for.

It is therefore a reasonable conclusion that there is only a small amount of information available relative to shallow surface layers in cold regions and vehicle performance.

Projects are now in progress to establish a comprehensive data bank over the next three years to correct this deficiency. Additional studies are planned to establish more accurate vehicle performance models for predicting mobility in winter environments.

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